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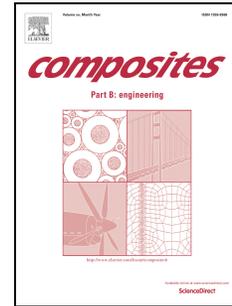
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Modified hinged beam test on steel fabric reinforced cementitious matrix (SFRCM)

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Abstract

An experimental campaign based on modified hinged beam test (MhBT) set-up has been reported in the present study. The samples consist of two concrete blocks coupled by a proper hinge device and laminated with steel wire fabrics embedded in a cementitious mortar layer. Two kinds of fabrics, made of galvanized steel strands with different mesh spacing, have been used to reinforce the concrete joists. With the aid of a DIC monitoring system, slippage profile at the interface between the concrete support and the mortar laminate along the contact region has been assessed, together with the fracture opening. Force vs slippage at the interface has been retrieved for the sampled tested according to the MhBT set-up. With the aim to obtain predictive ultimate load design formulas, a novel classification of laminate here proposed will be argued and related to a MhBT design formula. The influence of peel and shear stresses interaction on the ultimate strength of the system has been discussed in detail.

Keywords: FRCM, modified hinged beam test, steel fibres, slip profiles, fracture opening, ferrocement.

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1. Introduction

2 The use of both long and short fibres has known a wide development in
various engineering fields. This is mainly due to the high chemical and mechan-
4 ical fibres properties with respect to the same bulk materials form [1, 2]. In the
framework of aerospace and mechanical engineering a significant enhancement of
6 the base materials can be obtained by exploiting nano-tube technology [3, 4, 5].
Furthermore in the field of Civil Engineering, a basic and simple technology
8 together with a low manpower requirement have contributed to increase the
demand of fibre reinforced (FR) systems. In particular, fibre-based composites
10 are nowadays the most common material used as reinforcement and rehabili-
tation engagement on existing structures in the framework of civil engineering
12 [6, 7, 8, 9].

Indeed, continuous FR composite materials are commonly used as fibre re-
14 inforced polymers (FRP) and fabric reinforced cementitious matrix (FRCM).
In the case of reinforcements made of wire fabric embedded in a cementitious
16 matrix, the composite system is generally named "ferrocement". For the FRP
systems, almost always, the epoxy resin promotes the loading transfer between
18 the matrix and the fibres. Conversely, in FRCM, the cementitious matrix allows
the stress transfer to the embedded fibres.

20 Generally, FRP systems achieve higher mechanical performance with respect
to FRCM, but this leads to relevant drawbacks, typical of polymeric materials.
22 Fire resistance loss as well as lack of permeability, introduced by polymer ma-
trix, make FRP unreliable materials for some European cultural heritage au-
24 thority. Moreover, previous studies about the viscous effects in FRP [10] shown
that epoxy-concrete interface under constant load can exhibit displacement that
26 moving twice within few months. This relevant time-dependent effect, scoped
on structures subjected to cyclic or high temperature load [11, 12, 13, 14, 15]
28 can leads to a improper or useless repairing action. However FRCM, differently
from the FRP, are significantly affected by ageing strength loss under aggressive
30 environments which may reduce significantly the design limits [16].

Towards the end of the 20th century, different experimental tests have been
32 employed by the scientific community to investigate the bond-slip behaviour
of composite materials. In particular, the available shear test schedules are
34 subdivided in direct and indirect tests. Direct tests include the single lap set-
up [17, 18, 19, 20] (Figures 1(a) and 1(b)) and the double lap test layout [21]
36 (Figures 1(c) and 1(d)). Both single and double lap tests can be performed
in near end (NE) or far end (FE) version. On the other hand, the indirect
38 methods are the MhBT [7, 22], (Figure 1(e)) and the most common four points
bending test [23, 24], Figure 1(f). The indirect tests are characterized by a more
40 complex stress state with respect to the direct tests, which are more suitable for
the calibration of the design parameters. On the other hand, indirect methods
42 reproduce a stress state closer to that encountered in structural elements during
their life.

44 In order to connect the results provided by direct and indirect tests, a numer-
ical FE simulation based on non-linear fracture mechanics theory was performed
46 by [25], investigating the differences induced on the stress state by single, dou-
ble lap and beam tests. It's shown that, although of single and double lap
48 tests provide similar results, the MhBT provides higher ultimate bond strength
values.

50 The load transfer mechanism between the matrix and fabric plays a cru-
cial role in the design of composite structures. The investigation of the transfer
52 mechanism has led the scientific community to carry out extensive experimental
programs to better understand the interfacial stress distribution and its depen-
54 dence on the geometric and material parameters.

The experimental programs performed in literature pointed out that the
56 crack initiation at the nearest loading point shifts the active bonded zone toward
a new region farther from the loading point. The bond activation zone plays
58 a crucial rule, for which it has been introduced the concept of a characteristic
length, named effective or transfer length L_e , beyond which the load transfer
60 capability become negligible. A definition of the effective bond length as the
region in which the strain assumes a linear distribution was proposed in [22].

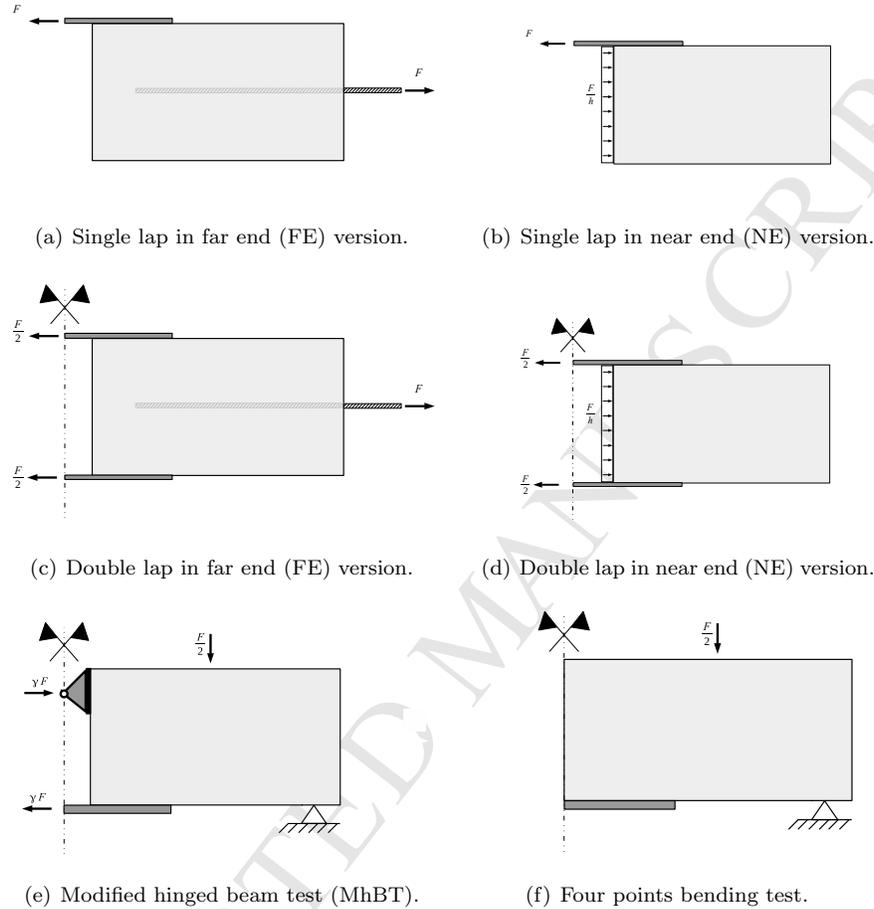


Figure 1: Available bond tests schedules.

62 Differently, in [26] the effective bond length was denoted as the length needed
 to attain the 97% of the ultimate load depending on the assumed bond-slip
 64 relationship provided by an assumed theoretical interface constitutive models.

As reported in [27], the monitoring of the shear tests by using conventional
 66 strain gauges does not provide sufficient information to grasp local phenomena
 and high strain levels which typically occur in the neighbourhood of the lam-
 68 inate edges. Conversely, the use of full-field monitoring methods are suitable
 to properly describe local effects. In particular, the constant growth of non-

70 destructive technologies based on optical physics for surface field monitoring
allows a higher resolution description of the delamination process. A far-field
72 airborne Radar (FAR) technique has been used in [28] to analyse the influence
of humidity and temperature on debonding phenomenon in FRP specimens
74 subjected to four points bending test. Digital image correlation (DIC) instru-
mentation allows monitoring the fracture opening in a NE single lap test to
76 capture the evolution of the interfacial stress distribution as the external load
increases [29]. Conversely, the use of full-field monitoring methods are suitable
78 to properly describe local effects. In particular, the constant growth of non-
destructive technologies based on optical physics for surface field monitoring
80 allows an higher resolution description of the delamination process.

The present paper deals with the bond behaviour of FRCM laminated on
82 a concrete support through a MhBT. The test program aims to the discussion
of the reliability, in terms of unwanted and encountered failure mode, of the
84 MhBT set-up. In addition, the possibility of this set-up to be performed on
specimens with reduced span length, with respect to the original structural ele-
86 ment, should be make this test particularly suitable for the bond test on existing
structural element. The test is performed by recording the load vs deflection,
88 under the upper knives, up to the sample failure. In order to investigate the
monitored fracture opening, for which an analytical treatment can be found in
90 [12, 30] and [31, 32], for infinitesimal and finite elasticity theory, respectively,
DIC monitoring has been used to investigate the slip at the interface as well as
92 fracture growth within the contact region. The paper is organized as follows:
Sec 2 describes the specimen and test schedule design based on the materi-
94 als characterization, as well as the test variables. The results are discussed
in Sec.3 in terms of force-displacement response and DIC post-processed slip
96 fields. The force-displacement result, supported by the DIC monitoring, allow
to understand the effect of lamination length in terms of global results, force-
98 displacement ultimate values and failure mode, as well as the local results, slip
profiles and fracture opening. A design formula has been used to predict the
100 ultimate load as a function of the failure mode strictly connected to the fab-

ric used. The influence of the peeling stress, generally neglected in theoretical
102 model, will be treated in Sec 4, by considering the analytical model of a beam
bonded to an elastic half-plane [33, 34].

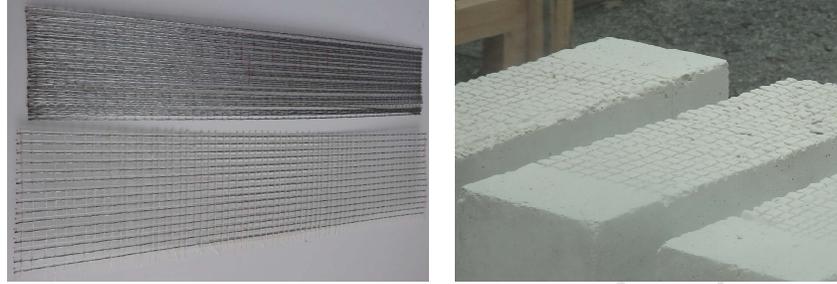
104 **2. Experimental program**

2.1. Research significance

106 The experimental program carried out in the present study has been orga-
nized to investigate the following issues:

- 108 • analyse the load transfer capability of two different types of unidirectional
steel fabrics (Figure 2(a)) laminated on a groove grid concrete support
110 (Figure 2(b));
- evaluate the capability of the MhBT to run as reference test for characteri-
112 zation of reinforcing and rehabilitation engagements on existing structures
owing to its wide adaptability both in terms of set-up arrangement and
114 in particular cases the sample number could be increased. Indeed start-
ing from two existing beam elements of 1.5m length, the MhBT allows
116 to carry out three tests instead of two, by the beams cutting into three
portions of 50cm length, in order to obtain three specimens realized from
118 the union of two 50cm beams block (as described in Section 2.2 and per-
formed in [7]). In addition, considering as an acceptable specimen block
120 length 50cm, the hinge presence, which length is about 8cm, allows to
earn an important length compared to the entire specimen. In the field of
122 the existing building structural element the sample number increase could
represent a relevant aspect. Particular attention will be paid to analyse
124 how vacuum thrust, an unwanted failure not encountered in the original
state of the manufacture, can influence the beam-test results;
- 126 • identify the serviceability load and the optimal lamination length over
which the load can be bear without exhibit crack patterns [30, 35];

- 128 • discuss the external work $\mathcal{L}_{e,\tau}$, related to the failure of the interface and defined as the area under the force-slip plot ($P - \delta$).



(a) From top to the bottom, wire fabric A and C (b) Surface groove grid

Figure 2: Steel fabric and surface treatment.

130 2.2. Experimental schedule and specimens design

A total of 24 specimens have been tested through MhBT schedule up to failure. Each specimen consists of two prismatic concrete blocks coupled by a mechanical hinge device placed at the upper part of the cross sections as shown in Figure 3(a). This schedule inspired to [36], born for longitudinal steel reinforcement bond test, is particular useful since both the compression and tension resultant magnitude and position are known during the test.

Two test variables have been considered. The first one is the lamination length (L_A), which denotes the length of the composite contact region for each block. The following effective bond length formulas, given by [23],

$$L_e = \sqrt{\frac{E_f t_f}{2f_{ctm}}} = 30 \text{ cm},$$

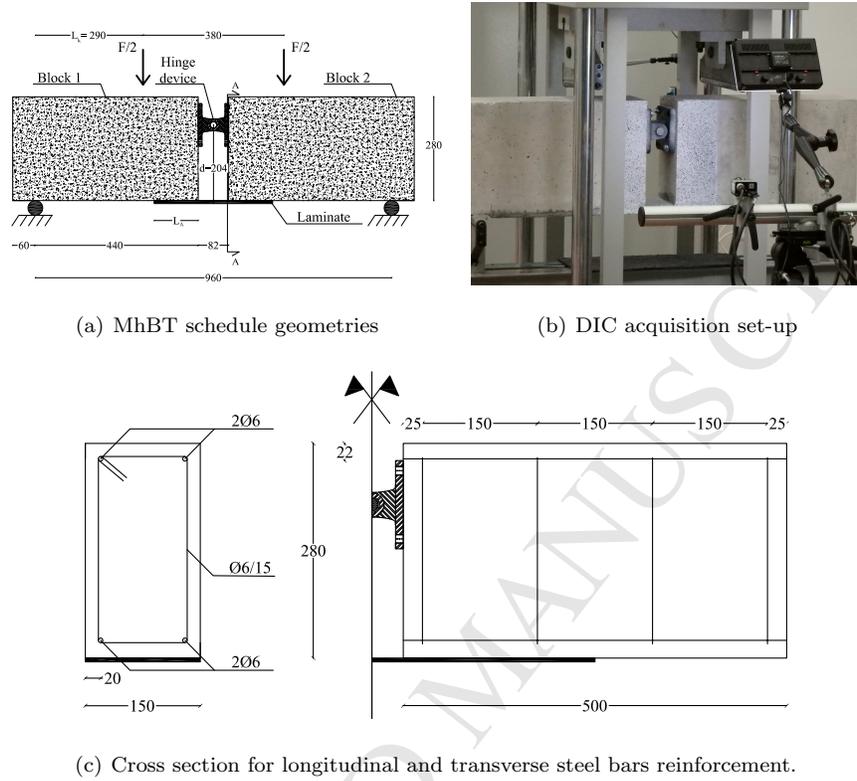
140 has been initially adopted to set the lamination length limits, where E_f , t_f and f_{ctm} are the fibre Young modulus, ultimate tensile mean strength (Table1) and the concrete ultimate tensile mean strength (Table2), respectively. It has been decided to investigate the behaviour of three different specimen groups for each type of wire fabric by choosing three different lamination lengths, equal

to 10, 20 and 30 cm. The second variable is represented by two unidirectional
146 wire fabrics characterized by different mesh spacings. These fabrics are made
of galvanized steel strands, named type A and C fabric, Figure 2(a), which
148 differ in the wire mesh spacing over the fabric section (A_f) fixed the support
width (b), as reported in Table (1). Each specimen has been labelled with a code
150 representative of fabric type, lamination length and identification number in the
specimen group (e.g A_{24} , represents the fourth specimen with 20 cm lamination
152 length made with wire fabric A).

The test set-up geometries aim to simulate elements representative of struc-
154 tural beams or slab joists sawed from an existing structure, as carried out in
[7]. All blocks have a rectangular cross section 15 cm width, 28 cm height (h)
156 and 50 cm long (L). During the concrete cast, each block has been reinforced
with longitudinal steel bars ($f_u = 450$ MPa) of 6 mm diameter and 2 cm cover,
158 placed at both upper and lower sides of the cross sections. Transverse steel
shear reinforcements with double bracket, 15 cm longitudinally spaced, have
160 been inserted in each concrete block, Figure 3(c).

In order to promote adhesion between the laminate and the substrate, after
162 14 days curing, a groove grating with a square mesh of 1.5×1.5 cm and 4 mm
depth, see Figure 2(b), has been realized on the lower surface of each block.
164 The specimens have been composed by coupling two blocks with a hinge device,
placed at the upper side of the cross section and fixed by means of proper
166 mechanical inserts, as shown in Figures 3.

Once the block pairs have been realized, the specimen intrados has been
168 wetted and a first thin cementitious mortar layer 4 mm height (t) was laid on
the block surface for a chosen lamination length. The steel fabric has been
170 applied on the mortar bed (still wetted) and the laminate has been completed
with a second mortar layer similar to the previous one. The attitude of the
172 composite layer to prevent delamination process is provided by the fabric wire
mesh, which confers higher internal grip to the laminate. After 28 days, once
174 the laminate has been cured, the specimens have been tested through a bending
test, according to Figure 3(a).



(a) MhBT schedule geometries

(b) DIC acquisition set-up

(c) Cross section for longitudinal and transverse steel bars reinforcement.

Figure 3: Test set-up, units [mm].

176 2.3. Materials characterization

In addition to the block pairs, six cubic samples of 15 cm size have been
 178 cast in order to measure the concrete compressive strength (R_{cm}) from which
 the concrete tensile strength (f_{ctm}) has been inferred (Table 2) according to
 [37]. In addition six $4 \times 4 \times 16$ cm prismatic samples of laminate cementitious

Type	ρ [g m ⁻²]	A_f/b [mm ² cm ⁻¹]	t_f [mm]	f_u [MPa]	ε_u [%]	E_f [GPa]
A	2000	4.72	0.084	2900	1.4	205
C	600	1.57				

Table 1: Galvanized steel fabric geometries and single strands mechanical properties.

	b [mm]	h [mm]	E [GPa]	R_{cm} [MPa]	f_{ctm} [MPa]
Laminate cementitious mortar	150	4+4	25	37.5(\pm 9.46)	8.86(\pm 5.27)
Concrete support	150	280	25	18.93(\pm 12.9)	1.88(\pm 8.64)

Table 2: Mortar laminate and concrete support properties.

mortar have been tested through three points bending test and compression
 182 test in order to assess the Young modulus (E), mean compressive strength and
 tensile strength (Table 2) according to [38]. The rough wire fabric, according to
 184 [39], has been characterized under tensile test, f_u , ε_u and E , i.e. the ultimate
 tensile stress, the ultimate strain and the Young modulus, respectively. The
 186 mean values provided by the experimental test have been reported in Table 1.

2.4. MhBT load and monitoring procedures

188 The laminated specimen pairs have been tested in a four-points bending
 test using a MetroCom 7170S02 machine equipped with a 200 kN load cell
 190 with 0.6 kN resolution. The test has been performed according to 1 mm/min
 displacement rate. The machine has been equipped with a LVDT (linear variable
 192 displacement transducer) to monitor the deflection at the loading point. For
 some specimens, the displacement field in the neighbourhood of the sample
 194 midspan has been acquired through a DIC instrumentation (Figure 3(b)) in
 order to grasp slip at the support-laminate or laminate layer interface. The
 196 specimens that have not been acquired with DIC have a slash symbol in Table 3
 under DIC amounts. DIC has been calibrated in order to acquire a 30×30 cm
 198 square window, centred at the middle span of the specimen. The acquisition
 system configuration ensures 10 μ m resolution.

200 In order to measure the substrate-laminate or laminate layers interface slip,
 DIC displacement field has been referred to horizontal reference lines L_1 and
 202 L_2 oriented with the x axis, as sketched in Figure 4, placed in correspondence
 of the first and second thin mortar laminate layer, respectively. During the

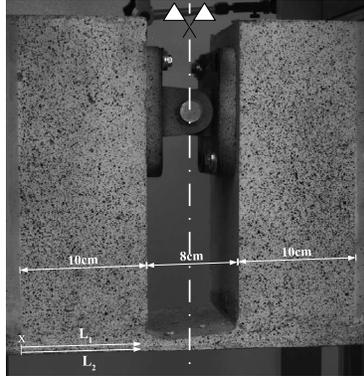


Figure 4: DIC reference system and reference lines.

204 loading phase, the gap between the horizontal displacements of the reference
 206 line provides the slip distribution along the sample. Note that a third reference
 208 line placed on the support over and parallel to L_1 should be required to evaluate
 laminate-substrate slip. Moreover, as discussed in Section 3, the groove grid
 realized on the support intrados before lamination provides a strong anchor
 system, hence no slip occurs at the support-laminate interface.

210 3. Results and Discussion

3.1. Force displacement results and failure modes

212 The MhBT results have been reported in Figure 5 for different types of wire
 fabrics. The detailed results for each type of wire fabric have been reported
 214 in Tables 3 and 4 in terms of the following quantities: maximum vertical force
 (F_{MAX}), vertical displacement corresponding to F_{MAX} (v_{MAX}), fibre maximum
 216 stress ($\sigma_{f,MAX} = P_{MAX}/A_f$), where P_{MAX} is the maximum tensile force on
 the fabric. It should be noted that the symbol F will be used in the following to
 218 denote the vertical resultant force applied by the upper knives, differently from
 the symbol P that denotes the force acting on the laminate evaluated as,

$$P = \frac{FL_k}{2d}$$

220 where L_k and d , represent distance between the upper and lower knives and the
 222 internal lever arm (Figure 3(a)). Moreover the overline symbol will be used to
 224 denote mean amounts of the entire specimens group. In addition, the DIC post
 226 processed data have been reported in Tables 3 and 4 in terms of maximum slip
 between the laminate layers (δ_u), external work ($\mathcal{L}_{e,\tau}$) and the relative error
 $\epsilon_{P_{MAX}}$ (defined from eq 3) for the estimation of the maximum tensile fibre force
 with respect to a theoretical predictive design formula eq 2.

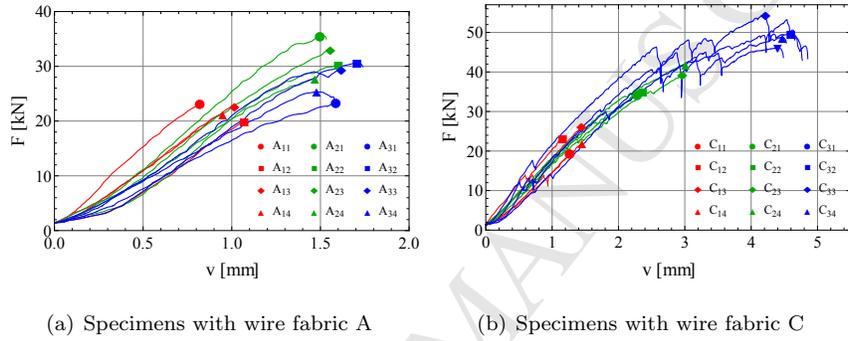


Figure 5: Beam-test force displacement results.

As reported in Figure 5 for specimens C, as the lamination length increases,
 228 the maximum load increases tending to a plateau. For the specimens C, the
 lamination length affects both the maximum strength and displacement. A
 230 different result has been provided by specimen A, where over the 20 cm lamination
 length, the maximum displacement are nearly the same while the maximum
 232 force decreases. Because of its larger mesh size, the wire fabric C provides high
 mechanical interaction between the laminate layers. As a consequence of the
 234 increase in shear transmission capability, both the maximum force and displace-
 ment increase with the lamination length, Figure 6. In the region close to the
 236 laminate edges the stress level grows with the external load. The stress redistri-
 bution, related to the longitudinal displacement field contour map in Figure 7,
 238 paints the activation of a more extended concrete reacting zone for specimen C
 with respect to the specimens A. A longer lamination length, for specimens C,
 240 implies a longer reacting region along the interface and, in turn, more compres-

L_A [mm]	Spec	v_{MAX} [mm]	F_{MAX} [kN]	σ_{fu} [MPa]	δ_u [mm]	$\mathcal{L}_{e,t}$ [kJ]
100	1	0.81	23.17	405.65	/	/
	2	1.07	20.03	350.55	/	/
	3	1.01	22.75	396.27	0.06	0.66
	4	0.95	21.28	372.49	0.10	0.78
200	1	1.53	35.55	622.26	/	/
	2	1.60	30.39	532.02	0.26	3.22
	3	1.56	32.97	577.12	0.64	5.59
	4	1.46	27.85	487.44	/	/
300	1	1.58	23.49	411.14	0.11	1.01
	2	1.73	30.62	535.92	0.13	2.35
	3	1.63	29.44	515.37	/	/
	4	1.58	25.34	443.64	0.16	2.66

Table 3: Results for wire fabric type A

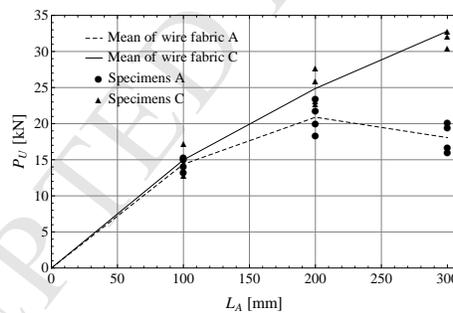


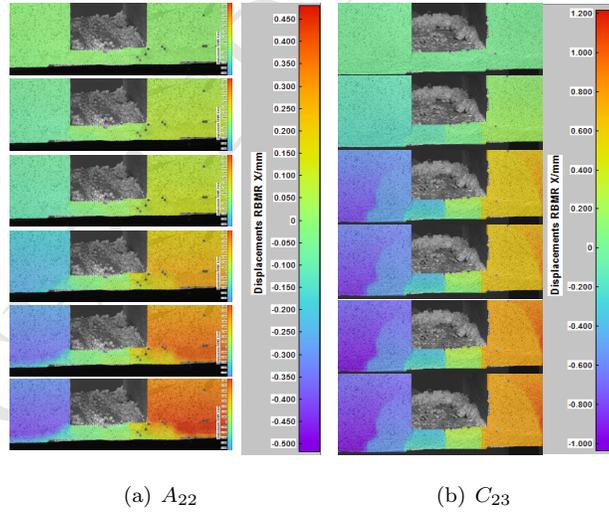
Figure 6: Effect of lamination length, maximum load with anchored length.

sive trusses inside the concrete samples, according to the Morsch scheme.

242 A summary of mean maximum load (\bar{F}_{MAX}), coefficient of variation (CV)
 and the mean fibre employment index, defined as the ratio between the mean fi-
 244 bre maximum stress over tensile fibre strength ($\bar{\sigma}_{f,MAX}/f_u$), have been reported
 in Table 5 for each specimen group. As highlighted by the employment index,
 246 fabric C achieved a stress level nearly close to its ultimate stress, conversely to

L_A [mm]	Spec	v_{MAX} [mm]	F_{MAX} [kN]	σ_{fu} [MPa]	ϵ_{P_u} [%]
100	1	1.26	19.38	1032.65	-19.29
	2	1.16	23.20	1235.73	-42.8
	3	1.45	26.25	1398.42	-61.58
	4	1.45	22.05	1175.00	-35.73
200	1	2.28	34.45	1835.07	5.75
	2	2.39	35.18	1874.11	3.76
	3	2.96	39.32	2094.55	-7.57
	4	3.01	41.99	2236.33	-14.87
300	1	4.77	49.73	2649.01	5.81
	2	4.85	54.45	2900.58	-3.13
	3	4.52	48.75	2596.82	7.67
	4	4.48	46.15	2458.22	12.59

Table 4: Results for wire fabric C

Figure 7: Longitudinal DIC displacement field u at different load levels, respectively 0.1, 0.4, 0.5, 0.85, 0.95 0.99 P_u .

Group	\bar{v}_u [mm]	\bar{F}_{MAX} [kN]	$\bar{\sigma}_{f,MAX}/f_u$
A_1	0.96 (± 11.6)	21.81 (± 6.58)	0.13
A_2	1.54 (± 3.84)	31.69 (± 10.46)	0.19
A_3	1.63 (± 4.34)	27.22 (± 12.35)	0.16
C_1	1.33 (± 10.86)	22.72 (± 12.53)	0.42
C_2	2.66 (± 14.23)	37.74 (± 9.42)	0.69
C_3	4.66 (± 3.92)	49.77 (± 6.96)	0.91

Table 5: Mean \pm CV [%] result and fibre strength optimization index.

the fabric A, in which the achieved stress is significantly lower.

248 Three different failure modes have been observed. An adhesive failure (AF)
 ([40]) has been found for the wire fabric A, Figures 8(a) and 8(b), owing to
 250 a progressive fracture propagation along the lower plane of the fabric. This
 type of failure is due to low mechanical grip induced by the small mesh size.
 252 Conversely, for specimens C failure occurred because of high stress level transferred
 from the laminate into the concrete support. In particular, in the case
 254 of C_1 specimens, failure was due to vacuum thrust (VT) generate by shearing
 stress in the neighbourhood of the block edge, with a consequent detachment
 256 of the concrete, as shown in Figure 8(d). Indeed, the concrete corner was subjected
 to stress concentration, and the increase of lamination length, for C_2 and
 258 C_3 group, has led to concrete shearing failure driven by crack formation and
 propagation starting from the laminate ends, Figures 8(e) and 8(f). For C_2
 260 specimens, crack propagation started from the laminate up to the intersection
 of the lower longitudinal steel reinforcement. Instead, for C_3 group, failure has
 262 been caused by crack propagation starting from the laminate which joins the
 compressive region. The higher load transfer capability of fabric C is evidenced
 264 by the load-displacement curve.

Each specimen C has exhibited a sudden loss and recovery in carry capacity
 266 before failure, as displayed in Figure 5. This response is related to progressive
 crack formations at the midspan of the laminate layer, highlighted as the lami-

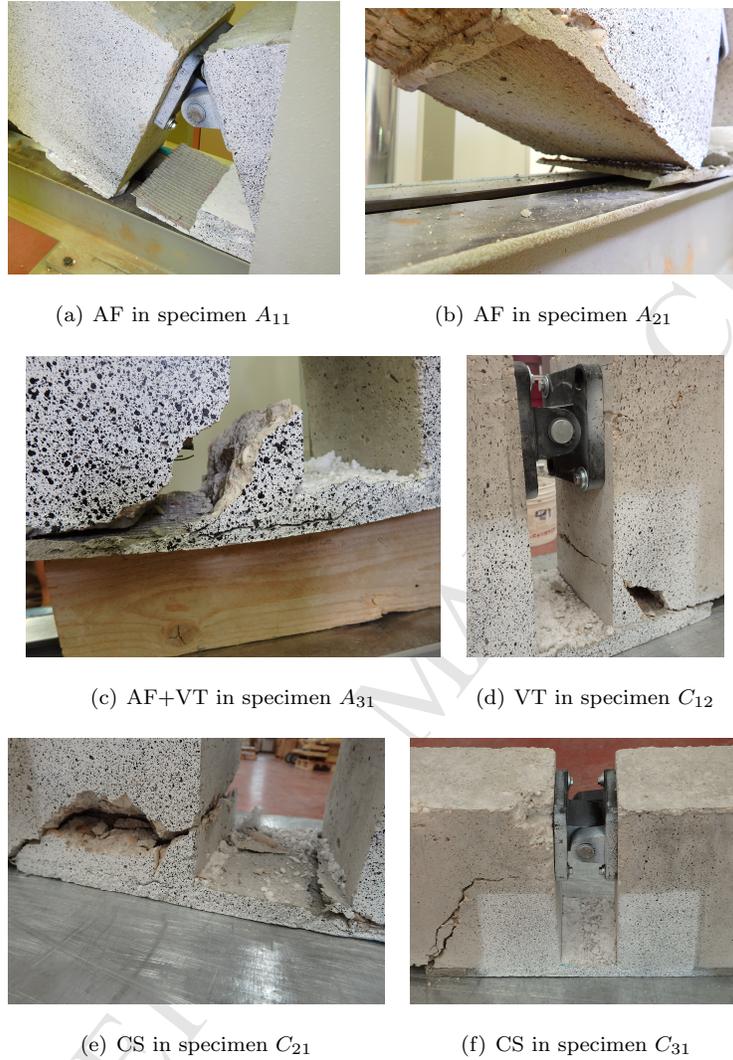


Figure 8: Failure mode: adhesive failure (AF), concrete shearing (CS) and vacuum thrust (VT).

268 nation length increase. This behaviour has not been observed in specimens A,
 where the system tends to dissipate energy through cracks propagation along
 270 the interface between fabric-mortar support.

3.2. Slip profiles, bond slip-relation and interface shear energy

272 Making reference to the system shown in Figure 4, the displacement compo-
 273 nent x along the L_1 and L_2 reference lines, respectively u_{L_1} and u_{L_2} , have been
 274 acquired by DIC optical system. As no cracks occur at the interface between
 support and laminate, only adhesive slip has been observed. Accordingly, the
 276 slip between the laminate layers has been calculated as follows

$$\delta(x) = u_{L_2}(x) - u_{L_1}(x).$$

The slip distribution has been assessed based on the difference of spline fitting
 278 on the displacement field provided by DIC along the aforementioned reference
 lines. The characteristic slip distributions have been reported in Figure 9 at
 280 different load levels for specimens A_{13} , A_{22} and A_{31} , representative of their own
 group behaviour. In addition, the slip profile of specimen A_{23} has been reported
 282 to better understand local phenomena observed and discussed in Section 3.3,
 as well as for two C specimens, to proof that no slip occurred for fabric C,
 284 Figures 9(e) and 9(f). For the sake of clarity, two vertical lines have been
 introduced in Figures 9 corresponding to the hinge ends. A stronger skew-
 286 symmetric slip profile shown by Figure 9(a) has been found for specimen A_{13}
 with respect to specimen A_{22} and A_{31} which shown skew trend in the initial
 288 load phases only, 9(b) and 9(d). For specimen A_{23} a different failure mechanism
 has been observed. The specimens C have not exhibited slip. As shown by
 290 Figures 9(e) and 9(f), the slip was caused by fracture phenomena localized
 in the neighbourhood of the blocks edges or near the hinge only. Conversely,
 292 for fabric A the slip increases with the load, reaching its maximum value in
 the neighbourhood of the block corners from which it propagates. For each
 294 specimen A, the maximum slip has been reported in Table 3.

In this study, slip has been computed as the relative displacement between
 296 L_1 and L_2 at the laminate lateral edge. Typically for the case of single lap test,
 the slip has been calculated by the use of LVDT placed at the upper surface
 298 of the laminate layer, where the acquired displacement became slip through
 the assumption of a fixed support. The lateral monitoring grasps exactly the

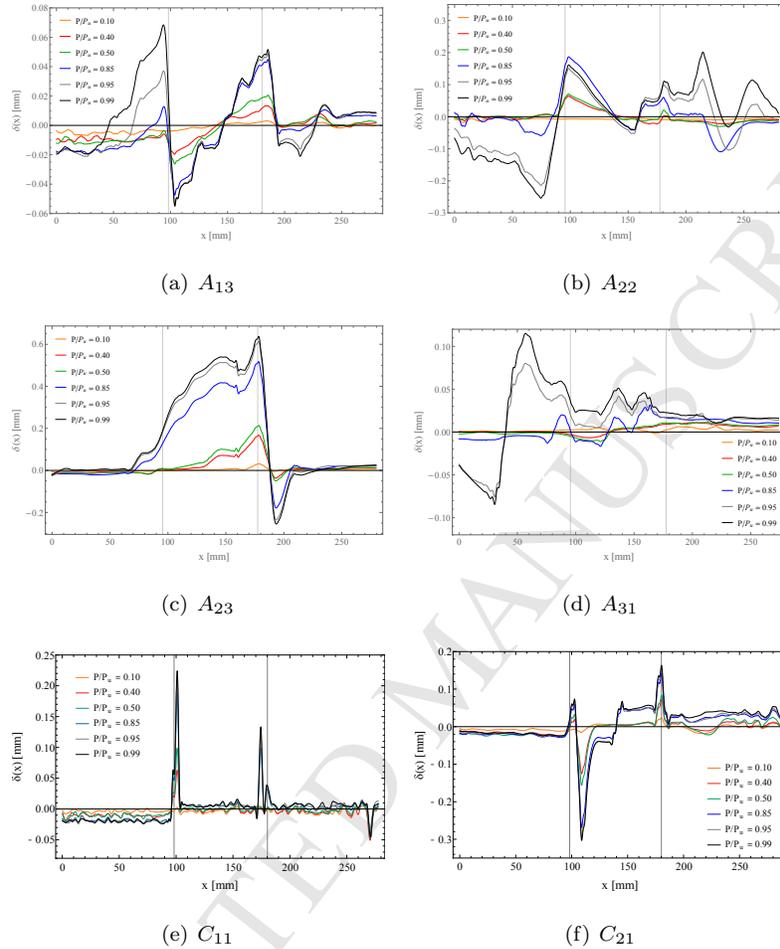


Figure 9: Slip profiles provided by DIC post-processing data referred to the reference lines.

300 relative slip profile definition but only on the lateral surface, while the upper
 surface slip monitoring allow to describe the slip profile anywhere. However
 302 this monitoring technique is incapable to distinguish laminate-support or inner
 laminate layers slip.

304 The local constitutive bond-slip relation affects the delamination phenomenon.
 Based on the assumptions of linear elastic behaviour of the concrete matrix,
 306 negligible laminate bending stiffness and peeling stress and assuming uniform
 distribution of the normal stress across the laminate cross section, the follow-

308 ing second order differential equation is found from the laminate and support
equilibrium and compatibility condition [26, 41],

$$\frac{d^2\delta(x)}{dx^2} - \Omega\tau(x) = 0, \quad (1)$$

310 where Ω represents the stiffness parameter of the system ($\Omega = \frac{1}{E_f t_f} + \frac{b_f}{b_s E_s t_s}$
or $\frac{2b_f}{E_f A_f}$ respectively from [26] and [41] where E_f , A_f and t_f represent the
312 Young modulus, cross section area and thickness of the fabric, respectively).
The determination of stress, strain and displacement fields are based on the
314 statement of a global $\tau(\delta)$ constitutive law. Different laws have been widely
investigate by [26] for FRP composite, starting from the assumption of linear,
316 multi-linear or multi-linear combined with exponential decay $\tau - \delta$ relations.

Differently from the direct tests, in the MhBT set-up the peeling stress
318 component becomes relevant and the approach proposed by [26] should be badly
grounded. So the external work has been calculated as a measure of the ductility
320 of the system,

$$\mathcal{L}_{e,\tau} = \int_0^{\delta_u} P d\delta.$$

The experimental force-slip curves provided by DIC post-processing have been
322 obtained fixing the x coordinate where the slip profile reaches its maximum
(Figure 9) with the associate monitored load absorbed by the laminate. The
force-slip curves have been reported in Figure 10. The obtained results show that

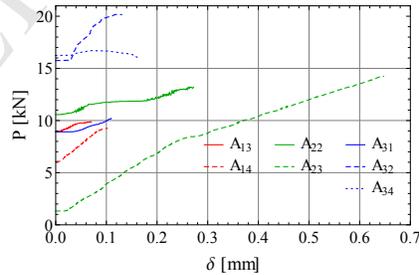


Figure 10: DIC bond-slip post-processing results.

324

the force increases almost linearly with the slippage, displaying an ascending

326 trend after an initial plateau. With particular attention to the specimen A_{22}
 and the singular A_{23} behaviour, despite of different values in the term of the
 328 ultimate displacement, the external work reproduces the same attitude of the
 system to dissipate energy.

330 3.3. Crack opening

For the case of specimen A, in order to establish a critical load threshold
 332 over which crack occurs at the interface between the laminate layers, the crack
 opening plots have been reported in Figure 11. These plots have been referred

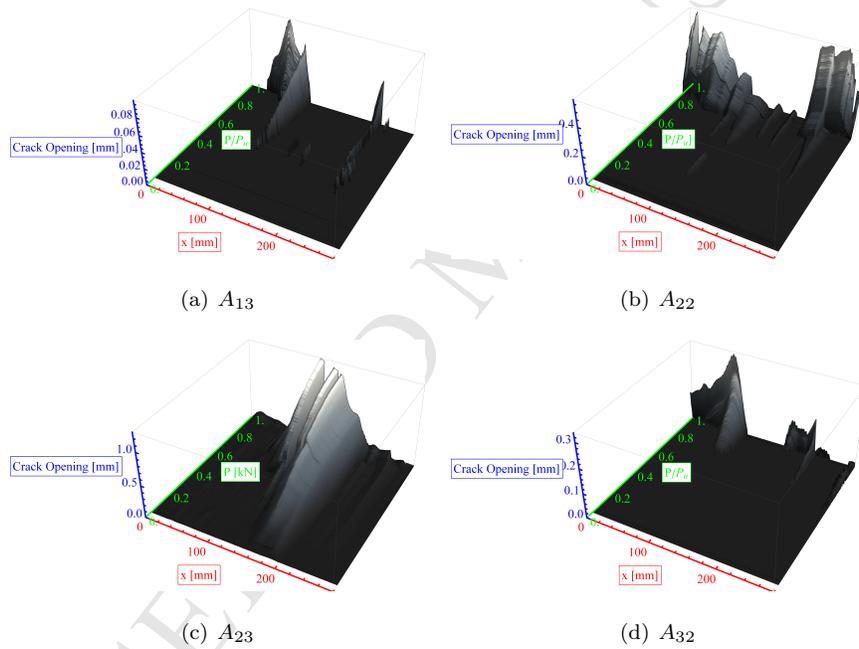


Figure 11: Crack propagation.

334 to the spatial coordinate system of Figure 4 and the ratio between the actual
 load and the ultimate load. The fracture opening has been obtained as the
 336 difference between the reference lines vertical displacements amongst which the
 fracture propagates,

$$v_{crack}(x, F) = v_{L_2}(x, F) - v_{L_1}(x, F).$$

338 With the exception of specimen A_{23} , an almost homogeneous and congruent
behaviour is retrieved for specimens belonging at the same group. As the lam-
340 ination length increases, the load threshold characterizing the crack formation
increases. For specimens A_{13} and A_{22} the effect provided by the lamination
342 length is more pronounced on the magnitude of the crack opening than the load
level corresponding to crack formation. The increase of dissipation capability
344 of the system is enforced by the external work grow, Table 3. Whereas for A_{13}
and A_{22} the crack propagation started at 0.5-0.6 P_u , for sample A_3 the critical
346 load shifted over 0.7 P_u . Moreover, for specimens A_3 the advantage provided
by increasing the lamination length in terms of crack formation is paid in terms
348 of dissipated energy which comes back closer to the values exhibited by sample
 A_1 . Differently from the previous cases, specimen A_{23} has shown a congruent
350 global behaviour with respect to its group, but showing an anomalous local
phenomenon. Indeed, sample A_{23} has exhibited a delamination process coupled
352 with a leveraging effect induced by the right block corner. Pushing of the right
block corner on the laminate induces an early crack formation. Figure 9(c)
354 shows that, for this specimen, slippage has occurred at higher magnitude and
earlier load level compared to the sample A_{22} . Slippage recovered for specimen
356 A_{23} is related to a counter-clockwise rotation induced by this phenomenon lo-
calized near the corners of the block. However, based on data listed in Table 3
358 and shown in Figure 10 it is remarked that, even if such a local phenomena
occurs, external work does not significantly change.

360 3.4. Design consideration

As shown in the previous Sections, a high strength and rigid interface, such
362 as the one provided by the fabric C, leads to a shearing failure in the concrete
support. The design of FRCM system bonded to a concrete support mainly
364 depends on the attitude of the laminate to transfer shear stress within two
interfaces: laminate-support and between laminate layers.

366 In the spirit of the Eurocodes classification for steel joints, an analogous dis-
tinction should be proposed for the laminate systems, divided into rigid systems,

368 partial-dissipative systems and and dissipative systems. The rigid FR systems
 induce failure of the support element without shear-slip dissipation. Conversely,
 370 the partial-dissipative FR systems are able to dissipate energy through shear
 interface energy but without inducing failure at the support. Finally, dissipative
 372 FR systems are characterized by a high shear interface energy dissipation which
 generates laminate detachment or debonding. With this proposed approach,
 374 specimens C belong to the rigid FR systems whereas specimens A are dissipa-
 tive systems. So the failure mode encounter in our experimental test is able to
 376 define the weak element of the FR system: support, laminate or laminate layers
 interface.

378 For the beam-test schedule, in the case of rigid FR systems, the design shear
 formula given by [37], for the shear traction strength criteria of beam element

$$V_{Rsd} = z \frac{A_{sw}}{s} f_{yd} [\cot(\alpha) + \cot(\theta)] \sin(\alpha)$$

380 has been used to predict the specimen C ultimate load for concrete shearing,
 where z , A_{sw} , s , f_{yd} , α and θ represent the internal lever arm, the transverse re-
 382 bars cross section area, the rebars span, the rebars yield design tensile strength,
 the angles of the rebars orientation and of the idealized compressive concrete
 384 element (by means of Morsch resistance criteria) with respect to the longitudi-
 nal bars respectively. The only change made on this formula is the substitution
 386 of the internal level arm value $z = 0.9d$ with

$$z = \min\{0.9d; L_A\}, \quad (2)$$

where d is the difference between the cross section height and the concrete cover.

388 As reported in Table 4, in terms of design formula relative error,

$$\epsilon_{P_{MAX}} = \frac{P_{u,M-NTC} - P_{MAX,test}}{P_{u,M-NTC}} \quad (3)$$

the ultimate load predicted by the rearranged [37] formula provides a self-
 390 confident results nearly always.

Conversely, for low strength matrix-fibre interfaces as for specimens A, dis-
 392 sipative FR system, [42] proposed an upper limit for the ultimate debonding

force, based on the shear energy computing,

$$P_{dbu} = \sqrt{2p_f E_f A_f G_f} \quad (4)$$

394 where E_f , A_f and G_f denote the Young modulus, section area and interface
fracture energy of the wire fabric, respectively, whereas $p_f = b$.

396 4. Peeling influence

The laminates ultimate forces P_u have been reported in Figure 6 with respect
398 to the lamination length. For the case of specimen A, the behaviour is really
close to some behaviours reported in [19, 42, 43, 44] for both FRPs and FRCMs
400 bonded to concrete support, but not argued. Our results, as well as some other
Literature results seem to suggest that over a certain lamination length the
402 ultimate force decreases ¹. This curious behaviour might be due to the complex
contact interface mechanical interaction between shear (τ) and peeling stresses
404 (q) investigated in the following.

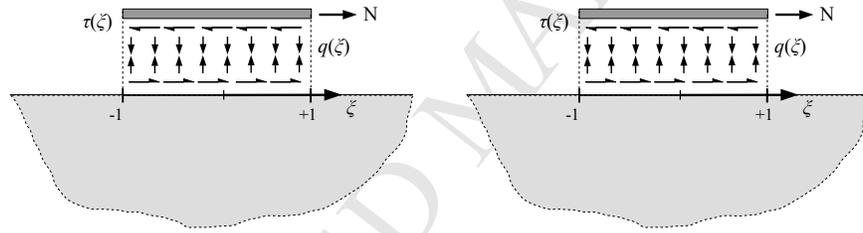
The mechanical interaction between a thin layer bonded to a substrate allows
406 understanding why the ultimate force of a composite bonded to a concrete
support tends to decrease over a certain lamination length. The analytical
408 studies performed in [33, 34] describe the mechanical interaction between a
cover and an elastic substrate taking into account both the peeling and shear
410 stresses $q - \tau$.

Let us consider the beam-substrate interaction representative of a thin lami-
412 nate bonded to a concrete support. The key point is to analyse how the stiffness
of the covering (laminated) with respect to the support stiffness can influence the
414 interface mechanical interaction. In particular, the superposition of two loading

¹The authors are aware that such behaviour can be attributed to a swing of data that,
in essence, would correspond to a plateau. But, recent MhBT and double lap tests, done
at lower displacement rate, as performed by [45], suggest that at higher displacement rate,
higher lamination lengths could reduce the system strength due to geometric aspects.

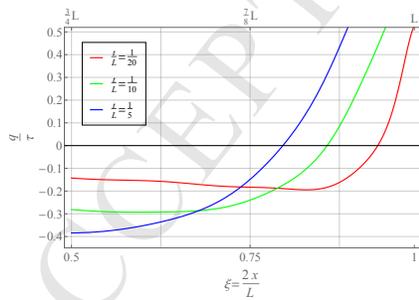
cases discussed in [34], namely a symmetric and skew-symmetric pair of axial
 416 forces acting at the beam edges, is significant to clarify this aspect.

The problem is governed by a dimensionless parameters $E_s \chi_l / G_l$ and the
 418 laminate slenderness ratio t/L , as observed by [34] yet, where χ and G are the
 shear factor and shear elastic modulus. The subscript l and s will be used to
 420 denote laminate and substrate quantities, respectively. The case of a single
 force acting at the right beam edge is reported in Figure 12(a). Hence, for
 422 this loading condition, the ratio between the interfacial peel and shear stresses
 has been reported in Figure 12(b) vs the dimensionless horizontal coordinate
 424 $\xi = x/L$ centred at the middle span of the laminate. In Figure 12(b), the
 darker grey areas represents the region in which the strength of the interface is
 promoted. Indeed, a positive peeling-tau ratio means that peeling component

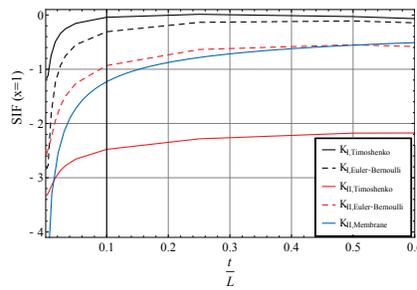


(a) Mechanical interaction reference system,
 with $E_s = E_l$, $\nu_s = \nu_l$.

(b) Interaction diagram



(c) Influence of laminate slenderness ratio in
 the neighbourhood of loading point



(d) Stress intensity factor for different model
 assumptions.

Figure 12: Mechanical interaction, peeling-shear stress ratio from [34] method, with $E_s = E_l$
 and $\nu_l = \nu_s = 0.21$.

plays a beneficial effect on the shear strength by supplying compression. Hence
 428 the laminate-substrate detachment, which always starts from the region closer
 to the loading point, turns up to be inhibited by the peeling stress component.

430 As the laminate slenderness ratio increases, the inversion point (IP), i.e. the
 point closer to the loading point where the mechanical interaction ratio (q/τ)
 432 changes in sign, moves to the left and, in turn, both the regions characterized by
 the maximum compressive peeling stresses increase, Figure 12(c). This means
 434 that, for high laminate slenderness ratios, the composite system increases sig-
 nificantly its debonding strength. Figure 12(c) highlights that despite of the
 436 commonly assumption of negligible peeling stress, the peeling stress component
 is able to achieve values higher than the 10% of the shear component. The ulti-
 438 mate load decreases as a critical lamination length is exceeded, thus suggesting
 that an optimal compromise between the materials strength and the q/τ interac-
 440 tion exists. As a consequence, once the geometry and the materials parameters
 (in particular, the laminate thickness, interface peeling and shear strength) are
 442 known as well the pure shear and peeling strength, the optimal bond length can
 be assessed. The analytical model allows evaluating how the laminate affects
 444 the failure mode of the system. By assuming three different models for the lami-
 nate, i.e. Timoshenko beam, Euler-Bernoulli beam and truss element, the stress
 446 intensity factors (SIFs) at the loading edge have been reported in Figure 12(d)
 varying t/L . As expected, the problem is governed by mode II, even if the mode
 448 I is not negligible with respect mode II.

5. Conclusions

450 An experimental investigation about beam-test on two different types of wire
 fabric with DIC monitoring has been performed. Through the post-processing of
 452 DIC data the specimen behaviour in terms of force-displacement curves, slippage
 distribution within the contact region and related energy as well as fracture
 454 opening have been discussed. The following conclusion can be drawn:

- the load transfer performance of wire fabrics A and C differ in terms of fail-

456 ure mode. Adhesive fracture has been observed for specimens with fabric
458 type A, which exhibit a pure friction strength mechanism based on slip-
page between the laminate layers, thus giving an almost linear ascending
or plastic force-slip curve. Conversely, specimens C could take advantage
460 of an extra mechanical strength provided by a high mesh spacing affecting
the employment index, as reported in Table 5;

- 462 • the vacuum thrust failure mode plays a crucial role only for sample C_1 .
This kind of failure indicates a good laminate-substrate shear transfer
464 attitude, which allow to achieve high performance by the increase of lam-
ination length. We suppose that, when vacuum thrust failure occurs, an
466 increase of the lamination length produces failures into the support, as
exhibited by samples C_2 and C_3 . Hence the MhBT set-up might be more
468 suitable to investigate the mechanical behaviour of a composite system in
which the lamination length exceeds the beam height minus the height
470 of the hinge device to avoid vacuum thrust failure. In order to better un-
derstand the results provided by MhBT compared with the other tests
472 schedules shown in Figure 1, a further experimental program based on
double lap and other beam-test, reducing the displacement rate, are in
474 progress;
- The results provided by fracture opening plot, Figure 11, suggest to use
476 a safety factor aimed at preventing the crack formation ranging between
0.5-0.7 (the serviceability load level). In addition for specimens C, the
478 modified Italian design formula [37], based on the common Morsch truss
strength mechanism provides a good design formula for beam-test results;
- 480 • the analytic model proposed by [34] highlights that fixed the elastic ma-
terial constants, the laminates with low values of slenderness promote the
482 debonding strength. This fact, as discussed in Section 4, confirms the
observed results, from which it can be state that over a certain lamination
484 length the ultimate force decreases;

- the proposed FR classification, i.e. rigid systems, partial dissipative systems and dissipative systems allows identifying the sacrificial element in order to obtain a given behaviour of the system. In addition, this classification could guide on design formula as a function of the governing failure mode.

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References

- [1] M. Gesoglu, E. Güneysi, G. F. Muhyaddin, D. S. Asaad, Strain hardening ultra-high performance fiber reinforced cementitious composites: Effect of fiber type and concentration, *Composites Part B: Engineering* 103 (2016) 74–83.
- [2] İ. Şanal, N. Özyurt, A. Hosseini, Characterization of hardened state behavior of self compacting fiber-reinforced cementitious composites (SC-FRCC's) with different beam sizes and fiber types, *Composites Part B: Engineering* 105 (2016) 30–45.
- [3] R. Barretta, F. M. De Sciarra, M. Diaco, Small-scale effects in nanorods, *Acta Mechanica* 225 (7) (2014) 1945–1953.
- [4] R. Barretta, F. M. de Sciarra, Analogies between nonlocal and local Bernoulli-Euler nanobeams, *Archive of Applied Mechanics* 85 (1) (2015) 89–99.
- [5] G. Romano, R. Barretta, M. Diaco, F. M. de Sciarra, Constitutive boundary conditions and paradoxes in nonlocal elastic nanobeams, *International Journal of Mechanical Sciences* 121 (2017) 151–156.

- 512 [6] L. Lanzoni, A. Nobili, A. Tarantino, Performance evaluation of a
polypropylene-based draw-wired fibre for concrete structures, *Construction*
514 *and Building Materials* 28 (1) (2012) 798–806.
- [7] A. Nobili, F. O. Falope, Impregnated carbon fabric–reinforced cementitious
516 matrix composite for rehabilitation of the Finale Emilia hospital roofs: Case
study, *Journal of composites for construction* 21 (4) (2017) 05017001.
- 518 [8] D.-Y. Yoo, N. Bantia, Y.-S. Yoon, Predicting service deflection of ultra-
high-performance fiber-reinforced concrete beams reinforced with GFRP
520 bars, *Composites Part B: Engineering* 99 (2016) 381–397.
- [9] A. Caratelli, S. Imperatore, A. Meda, Z. Rinaldi, Punching shear behavior
522 of lightweight fiber reinforced concrete slabs, *Composites Part B: Engineer-
ing* 99 (2016) 257–265.
- 524 [10] K. K. Choi, P. Meshgin, M. M. R. Taha, Shear creep of epoxy at the
concrete-FRP interfaces, *Composites Part B: Engineering* 38 (5) (2007)
526 772–780.
- [11] F. Germano, G. Tiberti, G. Plizzari, Experimental behavior of SFRC
528 columns under uniaxial and biaxial cyclic loads, *Composites Part B: Engi-
neering* 85 (2016) 76–92.
- 530 [12] C. Carloni, K. V. Subramaniam, Investigation of sub-critical fatigue crack
growth in frp/concrete cohesive interface using digital image analysis, *Com-
532 posites Part B: Engineering* 51 (2013) 35–43.
- [13] L. Dezi, A. M. Tarantino, Time-dependent analysis of concrete structures
534 with a variable structural system, *ACI Materials Journal* 88 (3).
- 536 [14] L. Dezi, G. Menditto, A. M. Tarantino, Viscoelastic heterogeneous struc-
tures with variable structural system, *Journal of Engineering Mechanics*
119 (2) (1993) 238–250.

- 538 [15] L. Dezi, G. Menditto, A. M. Tarantino, Homogeneous structures subjected
to repeated structural system changes, *Journal of Engineering Mechanics*
540 116 (8) (1990) 1723–1732.
- [16] A. Nobili, C. Signorini, On the effect of curing time and environmen-
542 tal exposure on impregnated carbon fabric reinforced cementitious matrix
(CFRCM) composite with design considerations, *Composites Part B: En-*
544 *gineering* 112 (2017) 300–313.
- [17] L. Bizindavyi, K. Neale, Transfer lengths and bond strengths for composites
546 bonded to concrete, *Journal of composites for construction* 3 (4) (1999)
153–160.
- 548 [18] T. D’ Antino, C. Carloni, L. Sneed, C. Pellegrino, Matrix–fiber bond be-
haviour in PBO FRCM composites: A fracture mechanics approach, *Engi-*
550 *neering Fracture Mechanics* 117 (2014) 94–111.
- [19] J. Yao, J. Teng, J. Chen, Experimental study on FRP-to-concrete bonded
552 joints, *Composites Part B: Engineering* 36 (2) (2005) 99–113.
- [20] F. Bencardino, A. Condello, A. F. Ashour, Single-lap shear bond tests on
554 steel reinforced geopolymeric matrix-concrete joints, *Composites Part B:*
Engineering 110 (2017) 62–71.
- 556 [21] M. R. Valluzzi, D. V. Oliveira, A. Caratelli, G. Castori, M. Corradi,
G. De Felice, E. Garbin, D. Garcia, L. Garmendia, E. Grande, et al., Round
558 robin test for composite-to-brick shear bond characterization, *Materials and*
Structures 45 (12) (2012) 1761–1791.
- 560 [22] L. De Lorenzis, B. Miller, A. Nanni, Bond of fiber-reinforced polymer lam-
inates to concrete, *ACI Materials Journal* 98 (3) (2001) 256–264.
- 562 [23] L. Ombres, Debonding analysis of reinforced concrete beams strengthened
with fibre reinforced cementitious mortar, *Engineering Fracture Mechanics*
564 81 (2012) 94–109.

- [24] A. Meda, F. Minelli, G. A. Plizzari, Flexural behaviour of RC beams in
566 fibre reinforced concrete, *Composites Part B: Engineering* 43 (8) (2012)
2930–2937.
- [25] J. Ghen, Z. Yang, G. Holt, FRP or steel plate-to-concrete bonded joints:
568 Effect of test methods on experimental bond strength, *Steel & Composite
Structures* 1 (2) (2001) 231–244.
- [26] H. Yuan, W. Wu, H. Yoshizawa, Theoretical solutions on interfacial stress
572 transfer of externally bonded steel/composite laminates., *Journal of Structural
Mechanics Earthquake Engineering I* (675) (2001) 27–39.
- [27] X. Lu, J. Teng, L. Ye, J. Jiang, Bond–slip models for FRP sheets/plates
574 bonded to concrete, *Engineering structures* 27 (6) (2005) 920–937.
- [28] O. Büyüköztürk, T.-Y. Yu, Understanding and assessment of debonding
576 failures in FRP-concrete systems, in: *Seventh International Congress on
Advances in Civil Engineering*, Istanbul, Turkey, Citeseer, 2006, pp. 1–22.
- [29] D. Corr, M. Accardi, L. Graham-Brady, S. Shah, Digital image correla-
580 tion analysis of interfacial debonding properties and fracture behavior in
concrete, *Engineering Fracture Mechanics* 74 (1) (2007) 109–121.
- [30] W. C. Choi, S. J. Jang, H. D. Yun, Bond and cracking behavior of
582 lap-spliced reinforcing bars embedded in hybrid fiber reinforced strain-
hardening cementitious composite (SHCC), *Composites Part B: Engineer-
584 ing* 108 (2017) 35–44.
- [31] A. M. Tarantino, Crack propagation in finite elastodynamics, *Mathematics
586 and Mechanics of Solids* 10 (6) (2005) 577–601.
- [32] A. M. Tarantino, Nonlinear fracture mechanics for an elastic Bell mate-
588 rial, *The Quarterly Journal of Mechanics and Applied Mathematics* 50 (3)
590 (1997) 435–456.

- 592 [33] P. Di Maida, F. O. Falope, Euler-Bernoulli nanobeam welded to a com-
pressible semi-infinite substrate, *Modelling and Simulation in Engineering*
2016 (2016) 4.
- 594 [34] L. Lanzoni, E. Radi, A loaded Timoshenko beam bonded to an elastic half
plane, *International Journal of Solids and Structures* 92 (2016) 76–90.
- 596 [35] K. Hannawi, H. Bian, W. Prince-Agbodjan, B. Raghavan, Effect of dif-
ferent types of fibers on the microstructure and the mechanical behavior
598 of ultra-high performance fiber-reinforced concretes, *Composites Part B:
Engineering* 86 (2016) 214–220.
- 600 [36] RILEM TC, RC 5 Bond test for reinforcement steel. 1. Beam test, E & FN
SPON, 1982.
- 602 [37] Ministero delle Infrastrutture, Consiglio Superiore dei Lavori Pubblici,
D.M. 14/01/08. Norme tecniche per le costruzioni (NTC2008), Italian Of-
604 ficial Bulletin 29.
- [38] UNI EN, Methods of test for mortar for masonry - Determination of flexural
606 and compressive strength of hardened mortar., UNI EN Standard.
- [39] ACI Committee 549.1R-93, Guide for the design, construction, and repair
608 of ferrocement, American Concrete Institute.
- [40] DT200 CNR, Guide for the design and construction of an externally bonded
610 FRP system for strengthening existing structures, Italian National Re-
search Council, Rome.
- 612 [41] A. D’Ambrisi, L. Feo, F. Focacci, Experimental and analytical investigation
on bond between carbon-FRCM materials and masonry, *Composites Part*
614 *B: Engineering* 46 (2013) 15–20.
- [42] A. D’Ambrisi, L. Feo, F. Focacci, Experimental analysis on bond between
616 PBO-FRCM strengthening materials and concrete, *Composites Part B:
Engineering* 44 (1) (2013) 524–532.

- 618 [43] J. Chen, J. Teng, Anchorage strength models for FRP and steel plates
bonded to concrete, *Journal of Structural Engineering* 127 (7) (2001) 784–
620 791.
- [44] T. Maeda, Y. Asano, Y. Sato, T. Ueda, Y. Kakuta, A study on bond
622 mechanism of carbon fiber sheet, in: *Non-Metallic (FRP) Reinforcement
for Concrete Structures, Proceedings of the Third Symposium, Vol. 1, 1997,*
624 pp. 279–286.
- [45] C. Carloni, S. Verre, L. H. Sneed, L. Ombres, Loading rate effect on the
626 debonding phenomenon in fiber reinforced cementitious matrix-concrete
joints, *Composites Part B: Engineering* 108 (2017) 301–314.